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CE 431: Highway Engineering Tutorial Note 6: Chapter 15 Eng. Ibrahim Almohanna

15-1 Given the following information, calculate the group index and classify each of these soils into proper subdivision of AASHTO classification system.

| Sieve Analysis | | | Characte | ristics of Soil | |
|----------------|--------------------|------------------|-------------------|-----------------|---------------------|
| Sample . | Percentage Passing | | Binder | | |
| No. | 10 (2.00 mm) | 40 (0.425 mm) | 200 (0.075 mm) | Liquid Limit | Plasticity Index |
| 1 | 100.0 | 97.5 | 65.1 | 65.5 | 45.0 |
| 2 | 100.0 | 73.1 | 5.4 | 20.1 | NP |
| 3 | 100.0 | 46.2 | 20.8 | 16.9 | NP |
| 4 | 77.2 | 37.1 | 28.2 | 33.1 | 6.8 |
| 5 | 100.0 | 100.0 | 58.2 | 60.2 | 21.7 |
| 6 | 100.0 | 100.0 | 83.7 | 54.2 | 33.6 |
| 7 | 100.0 | 56.3 | 19.1 | 24.8 | 12.6 |
| 8 | 100.0 | 96.3 | 75.6 | 33.7 | 8.9 |
| 9 | 100.0 | 100.0 | 53.1 | 50.6 | 8.6 |
| 10 | 100.0 | 100.0 | 95.0 | 30.3 | 11.3 |
| 11 | 100.0 | 73.7 | 36.3 | 38.7 | 12.7 |
| 12 | 37.1 | 21.1 | 8.6 | 12.3 | NP |

Group Index values are calculated by equation 15-4, and the AASHTO classifications determined by Table 15-1 using a left-to-right elimination process.

| Sample No. | Group Index | AASHTO Classification |
|------------|-------------|-----------------------|
| 1 | 45 | A-7-6 |
| 2 | 0 | A-3 |
| 3 | 0 | A-1-b |
| 4 | 0 | A-2-4 |
| 5 | 12 | A-7-5 |
| 6 | 29 | A-7-6 |
| 7 | 0 | A-2-6 |
| 8 | 6 | A-4 |
| 9 | 4 | A-5 |
| 10 | 11 | A-6 |
| 11 | 1 | A-6 |
| 12 | 0 | A-1-a |

15-2 Classify each of the soils in Problem 15-1 under the Unified Soil Classification System. The following additional data are available for certain of the samples.

| Sample No. 2 | D ₆₀ = 0.3, D ₁₀ = 0.03, D ₃₀ = 0.17 | | |
|---------------|---|--------|--|
| Sample No. 4 | Passing No.4 sieve | 86.2% | |
| | Passing 3/8-in. sieve | 100.0% | |
| Sample No. 12 | Passing No.4 sieve 49 | | |
| | Passing ½ -in. sieve | 80.6% | |
| | Passing 1-in sieve | 100.0% | |
| | C _u = 5 | | |
| | C _g = 2 | | |
| | | | |

Using Table 15-3, the following classifications result:

| Sample No. | Unified Soil Classification System |
|------------|------------------------------------|
| 1 | СН |
| 2 | SP |
| 3 | SM |
| 4 | SM or SC |
| 5 | OH or MH |
| 6 | СН |
| 7 | SM or SC |
| 8 | ML |
| 9 | OH or MH |
| 10 | CL |
| 11 | SM or SC |
| 12 | GW |

15-3 Given the following information from a compaction test performed in a laboratory by the standard Proctor compaction procedure, draw the moisture-density curve and determine the optimum moisture and maximum density for this soil:

Weight of mold = 2456 g Volume of mold = 1/30 ft³

| Trial No. | Weight of Compacted Soil Plus Mold (g) | Moisture Content (%) |
|-----------|---|-------------------------|
| 1 | 4136 | 9.4 |
| 2 | 4205 | 11.2 |
| 3 | 4308 | 13.1 |
| 4 | 4408 | 13.9 |
| 5 | 4398 | 15.8 |
| 6 | 4354 | 17.9 |

Sample calculation for trial No. 1:

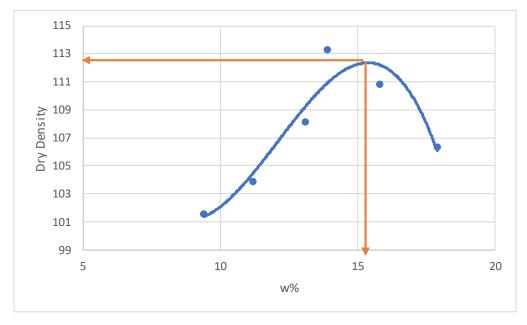
$$Wt. of \ soil = Wt. of \ soil \ and \ mold - Wt. of \ mold = 4136 - 2456 = 1680 \ g$$

$$Wt. of \ soil \ in \ (lb.) = 1680 \ g \times \frac{1lb.}{453.59 \ g} = 3.704 \ lb.$$

$$wet \ unit \ Wt. = \frac{wet \ Wt. \ lb.}{volume \ ft^3} = \frac{3.704}{1/30} = 111.1 \ \frac{lb.}{ft^3}$$

$$dry \ unit \ Wt. = \frac{wet \ unit \ Wt.}{\frac{(100 + w\%)}{100}} = \frac{111.1}{\frac{(100 + 9.4)}{100}} = 101.6 \ \frac{lb.}{ft^3}$$

| Trial No. | Wt. of Soil + Mold (g) | Wt. of Soil (g) | Wt. of Soil (lb.) | Wet Unit Wt. (lb./ft³) | w % | Dry Unit Wt. (lb./ft³) |
|-----------|---------------------------|--------------------|----------------------|---------------------------|------|---------------------------|
| 1 | 4136 | 1680 | 3.704 | 111.1 | 9.4 | 101.6 |
| 2 | 4205 | 1749 | 3.856 | 115.5 | 11.2 | 103.9 |
| 3 | 4308 | 1852 | 4.083 | 122.4 | 13.1 | 108.2 |
| 4 | 4408 | 1952 | 4.303 | 129.0 | 13.9 | 113.3 |
| 5 | 4398 | 1942 | 4.281 | 128.4 | 15.8 | 110.9 |
| 6 | 4354 | 1898 | 4.184 | 125.4 | 17.9 | 106.4 |



Optimum w% = 15.5%,

maximum dry density = 12.5 lb./ft³.

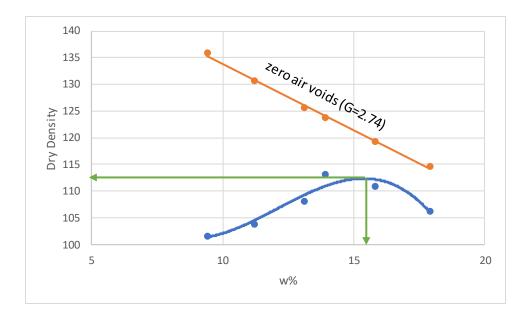
15-4 Assuming the soil of Problem 15-3 has a specific gravity of 2.74, make the necessary computations and plot the "zero air voids curve" on the drawing prepared in Problem 15-3.

Using equation 15-5, a sample calculation for trial No. 1:

dry unit Wt. =
$$\frac{wG}{1 + \frac{mG}{100}} = \frac{62.4 \times 2.74}{1 + \frac{9.4 \times 2.74}{100}} = \frac{170.976}{1.25756} = 136.0 \frac{lb.}{ft^3}$$

| Trial | w % | Dry Unit Wt. |
|-------|------|--------------|
| No. | W 70 | (lb./ft³) |
| 1 | 9.4 | 136.0 |
| 2 | 11.2 | 130.8 |
| 3 | 13.1 | 125.8 |
| 4 | 13.9 | 123.8 |
| 5 | 15.8 | 119.3 |
| 6 | 17.9 | 114.7 |

5



15-5 The dry mass of a sample of aggregate is 1206 g. The mass in a saturated surface dry condition is 1226.8 g. The volume of the aggregate, excluding the volume of absorbed water, is 440.6 cm³. Calculate the bulk specific gravity, and the percentage absorption.

By equation 15-7, the bulk specific gravity

Mass of absorbed water = 1226.8 - 1206 = 20.8 g

Volume of absorbed water = 20.8 g / 1 g/cm³ = 20.8 cm³

Volume of aggregate with water = $440.6 + 20.8 = 461.4 \text{ cm}^3$

$$G_B = \frac{M_D/V_B}{W} = \frac{1206(g)/461.4(cm^3)}{1(\frac{g}{cm^3})} = 2.614$$

By equation 15-7, the percentage absorption:

Percentage absorption =
$$\frac{M_w}{M_D} \times 100 = \frac{20.8}{1206} \times 100 = 1.72\%$$

15-6 The following weights are recorded during the determination of the specific gravity of bituminous material by pycnometer method. Calculate the specific gravity of this substance.

| Weight of pycnometer, empty | 34.316 g |
|---|---------------|
| Weight of pycnometer, filled with water | 60.000 g |
| Weight of pycnometer, filled with bituminous material | 58.202 g |
| Temperature (all determinations) | 77 °F (25 °C) |

$$G = \frac{C - A}{(B - A) - (D - C)}$$

Where:

A = Weight of pycnometer.

B = Weight of pycnometer filled with water.

C = Weight of pycnometer partially filled with asphalt, and

D = Weight of pycnometer plus asphalt plus water.

Since the pycnometer is filled with bituminous material, then D=C

$$G = \frac{58.202 - 34.316}{(60.000 - 34.316) - (58.202 - 58.202)} = 0.930$$